

**Exhibit X. The Lakes at Madison Park Site
Preliminary Geotechnical Engineering Report**

June 15, 2015

Darby Holdings, LLC
810 Main Street
Madisonville, Louisiana 70447

Attn: Ms. Judy Darby

Re: Geotechnical Engineering Report
General Site Characterization
Judy Darby Site
St. Tammany Parish, Louisiana
Project No. G15-044

Dear Ms. Darby:

Stratum Engineering, LLC (SE) is pleased to submit our Geotechnical Engineering Report for the above referenced project. The report includes the results of field and laboratory testing, as well as recommendations regarding the suitability of the site for future industrial developments.

We appreciate the opportunity to perform this geotechnical study and look forward to continued participation during the design and construction phases of this project. If you have any questions pertaining to this report, or if we may be of further service, please contact our office.

Respectfully submitted,
STRATUM ENGINEERING, LLC



William "Dean" McInnis, E.I.
Project Manager

WDM/TYM:wdm



Tony Y. Maroun, P.E.
Principal



GEOTECHNICAL ENGINEERING REPORT

**GENERAL SITE CHARACTERIZATION
JUDY DARBY SITE
ST. TAMMANY PARISH, LOUISIANA**

SE PROJECT NO. G15-044

PREPARED FOR

**DARBY HOLDINGS, LLC
C/O MS. JUDY DARBY
810 MAIN STREET
MADISONVILLE, LOUISIANA 70447**

JUNE 15, 2015

BY

**STRATUM ENGINEERING, LLC
585 JOHNNY F. SMITH AVENUE
SLIDELL, LOUISIANA 70460**

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EXECUTIVE SUMMARY

An exploration of the subsurface conditions has been completed at the Judy Darby Site in St. Tammany Parish to assess its suitability for potential future industrial developments.

The site encompasses about 34 acres of undeveloped property located just south of Interstate 12 near LA Highway 1077 in St. Tammany Parish. The site has a large detention pond on the north end while the rest of the property is a mixture of light and heavily wooded areas. The surface is covered with surface vegetation and brush. Topographic information was not available to us during this study. It is understood that the site will be marketed for industrial type developments. Typical facilities could be of structural steel frame or cast-in-place concrete.

Based on the borings, about 8 inches of silty sandy topsoil with organics covered the surface. The topsoil was underlain by dense sandy silt or very stiff sandy silty clay extending to about 2 feet. The silty material was generally followed by stiff to very stiff lean clay or sandy lean clay extending to a depth of about 12 to 15 feet. At that depth, a very stiff fat clay layer was encountered to about 47 feet and followed by stiff sandy lean clay to approximately 57 feet. The sandy lean clay was underlain by very stiff fat clay extending to the maximum depth explored at 50 feet. Groundwater ranged from 3 ½ to 7 feet during drilling.

Based on the field data and laboratory test results, the soils encountered near the surface consist mostly of dense sandy silts or very stiff sandy silty clays. The near surface silty soils encountered at the site are generally stable when dry. However, they are moisture sensitive and can lose their support capabilities if they become saturated. Therefore, depending on the site condition at the time of construction, the silty soils may have to be removed and replaced with compacted structural fill.

Provided the site is prepared as recommended, the soils at the site are generally suitable for supporting the structures on shallow foundations with floor slabs on grade. Although the soil condition at the site is fair, the foundation type will also depend on the type of structure and the magnitude of the structural loads.

The recommendations provided in this report are preliminary in nature and were formulated based on anticipated loading conditions for typical industrial structures. Furthermore, the recommendations were based on a few borings drilled across the site at accessible locations. Therefore, additional borings will be necessary when the type of facilities are identified and the locations of the structures are finalized to verify the soil conditions and provide final site specific recommendations for the structures.

The owner/designer should not rely solely on this Executive Summary and must read and evaluate the entire contents of this report prior to utilizing our engineering recommendations in preparation of design/construction documents.

PROJECT INFORMATION

Project Authorization

Stratum Engineering, LLC (SE) has completed a preliminary geotechnical exploration to characterize the Judy Darby Site for a potential future development in St. Tammany Parish, Louisiana. The exploration was accomplished in general accordance with SE Proposal No. G15-050R, dated March 20, 2015 and revised May 6, 2015.

Project Description

The property is located along the I-12 corridor in an area of prime industrial/business developments. In anticipation of comparable business or industrial developments on this property, the site will be characterized to verify the soil conditions and provide preliminary foundation recommendations for typical structures which could be constructed at the site.

Generally, industrial developments could consist of multiple structures with associated light and heavy duty pavements. The buildings may be single story structures with steel frames and load bearing masonry or tilt up walls, or could be of cast-in-place concrete. Depending on the building spans, maximum interior column load could range from 100 to 150 kips. Maximum wall loads are assumed to be 3 to 4 kips per foot. Depending on the facility type, floor loads could range between 250 to 650 psf. The facilities may be dock high, requiring 4 to 5 feet of fill to reach the building finished floor elevation.

Traffic associated with industrial facilities could consist of heavy tractor trailers with an average daily traffic (ADT) of 100 to 300 trucks per day for a design life of 20 years. For these types of facilities, rigid pavements are widely considered for their longevity and ability to support the high volume of traffic.

Purpose and Scope of Services

The purpose of this study was to explore the subsurface conditions at the site and to enable an evaluation of a cost effective foundation system for potential future industrial facilities.

The scope of services included drilling one (1) boring to a depth of 70 feet and two (2) borings to a depth of 20 feet at accessible locations across the site. In addition to drilling the soil borings, our scope of services included a reconnaissance of the project site, select laboratory testing, and preparation of this geotechnical report. This report briefly outlines the testing procedures, presents available project information, describes the site and subsurface conditions, and provides results of analysis and recommendations regarding the following:

-
- Preliminary foundation type, depths, allowable bearing capacities, and estimate of settlements;
 - Seismic site classification;
 - Typical soil parameters for flexible and rigid pavements;
 - Site preparation, including subgrade preparation and fill compaction requirements.

The scope of geotechnical services did not include an environmental assessment for determining the presence or absence of wetlands, or hazardous or toxic materials in the soil, surface water, groundwater, or air on or below, or around this site. Any statements in this report or on the boring logs regarding odors, colors, and unusual or suspicious items or conditions are strictly for informational purposes.

SITE AND SUBSURFACE CONDITIONS

Site Description and Location

The site encompasses approximately 33.54 acres of undeveloped property located adjacent to Interstate 12 on the south side. Access to the property is made from Vista Street to the south through a gated drive. The property is mostly cleared with several existing pathways bisecting the area. Several boats, trailers, and other pieces of equipment are currently stored near the south end of the property. The site is covered with surface vegetation and sparse trees with the exception of a heavily wooded portion on the north side along Interstate 12 adjacent to a large existing detention pond.

Detailed grading information was not available at the time this report was prepared. However, it was assumed that two (2) to 3 feet of fill may be needed to reach the design grade.

Field Exploration

The field exploration included a reconnaissance of the project site, drilling the required soil test borings and recovering undisturbed soil samples. Level of groundwater encountered in the soil borings was also measured and recorded.

The number of borings was suggested by CSRS, the Project Managers for the site, to support the Louisiana Economic Development (LED) Site Certification process. A total of three (3) borings were drilled to depth of 20 to 70 feet below the existing ground surface. The boring depths are in reference to the existing ground surface at the time of the field exploration. The borings were located in the field by Stratum Engineering and Darby Holdings representatives at accessible locations across the site. The approximate locations of the borings are indicated on a plan included in the Appendix. The drawing is a reproduction of an aerial photograph of the property.

Drilling and Sampling

The borings were drilled with an All-Terrain Vehicle (ATV) mounted drilling rig. Auger and wet rotary drilling techniques were used to advance the borings. Samples were generally obtained continuously from the ground surface to a depth of ten feet and at maximum five foot intervals thereafter. Drilling and sampling techniques were accomplished in general accordance with ASTM Standards.

Undisturbed samples of cohesive soils were generally obtained using thin-wall tube sampling procedures in general accordance with the procedures for “Thin-Walled Tube Geotechnical Sampling of Soils” (ASTM D1587). These samples were extruded in the field with a hydraulic ram and were wrapped in aluminum foil prior to placement in a plastic wrapping to preserve moisture. The samples were transported to the laboratory in containers to prevent disturbance.

The laboratory testing program included supplementary visual classification and water content tests on all of the soil samples. In addition, selected samples were subjected to unconfined compression testing, percent passing the #200 sieve and Atterberg Limits determination. Additional estimates of unconfined compressive strength were made using a hand penetrometer. The laboratory testing was performed in general accordance with ASTM Standard Procedures.

Subsurface Conditions

Based on the borings, about 8 inches of silty sandy topsoil with organics covered the surface. The topsoil was underlain by dense sandy silt or very stiff sandy silty clay extending to about 2 feet. The silty material was generally followed by stiff to very stiff lean clay or sandy lean clay extending to a depth of about 12 to 15 feet. At that depth, a very stiff fat clay layer was encountered to about 47 feet and followed by stiff sandy lean clay to approximately 57 feet. The sandy lean clay was underlain by very stiff fat clay extending to the maximum depth explored at 50 feet.

The above subsurface description is of a generalized nature to highlight the major subsurface stratification features and material characteristics. The boring logs included in the Appendix should be reviewed for specific information at the boring locations. These records include soil descriptions, stratification, penetration resistances, and locations of the samples and laboratory test data. The stratification shown on the boring logs represent the conditions only at the actual boring locations. Variations may occur and should be expected between boring locations. The stratification represents the approximate boundary between subsurface materials and the actual transition may be gradual. Water level information obtained during field operations is also shown on the boring logs. The samples, which were not altered by laboratory testing, will be retained for 60 days from the date of this report and then will be discarded.

Groundwater Conditions

Groundwater was encountered at depths ranging from 3 ½ to 7 feet during the drilling operations. It should be noted that groundwater levels will fluctuate with seasonal variations in rainfall, extended periods of drought or surface runoff. Therefore, it is recommended that the actual groundwater level at the site be determined by the contractor at the time of the construction activities.

IBC Site Classification

The International Building Code (IBC), 2012 edition, was reviewed to determine the site classification for seismic design. Based on the soils encountered in the borings and our experience in the general vicinity, the site can be classified as Site Class “D”, as outlined in Section 1613.3.2 of the building code.

EVALUATION AND RECOMMENDATIONS

General

It is our understanding the site will be marketed for potential industrial development. Typical structures could be dock high facilities requiring 4 to 5 feet of fill to reach the design grade. Otherwise, single story structures with slab-on-grade could require about 2 to 3 feet of fill to achieve the design grade.

The results of the exploration indicate that the near surface soils present at this site are generally adequate to support typical industrial facilities on a shallow foundation system. However, the recommendations provided are preliminary in nature and were formulated based on assumed loading conditions for typical industrial structures. Consequently, additional borings will be necessary when the type of facilities are identified and the locations of the structures are finalized to verify the soil conditions and provide final site specific recommendations for the structures.

Site Preparation

Site preparation is expected to include, but not be limited to, stripping of the topsoil, organics and other deleterious materials, and removal of any soft material encountered in the building and parking areas. Based on the borings, up to 8 inches of silty sandy topsoil with organics was encountered at the site. However, the actual stripping depth should be determined by a representative of the Geotechnical Engineer at the time of construction.

The borings were conducted at accessible locations across the site, and the surface was relatively dry and stable. However, the silty soil encountered near the surface is moisture sensitive and could lose its strength if saturated with water. Depending on the site condition at the time of construction, about 12 to 24 inches of the near surface moisture sensitive soil may have to be undercut prior to fill placement.

The subgrade in the building and pavement areas should be proofrolled with a tandem axle dump truck or a similar heavily loaded rubber tired vehicle weighing 15 to 20 tons. Soils which are observed to rut or deflect excessively under the moving load should be undercut and replaced with properly compacted structural fill. The proofrolling and undercutting activities should be witnessed by a representative of the Geotechnical Engineer and should be performed during a period of dry weather.

After subgrade preparation and observation have been completed, structural fill placement may begin. The fill should consist of silty sand, sandy clay or clayey sand and should be free of organics or other deleterious materials. The fill should have a liquid limit less than 40 and a plasticity index less than 18 percent. Structural fill soils with plasticity indices in this range will require close moisture content control to achieve the recommended degree of compaction. The structural fill should be compacted to at least 95 percent of the fill's maximum dry density as determined by ASTM D698 (Standard Proctor).

The fill should be placed in maximum lifts of 8 inches of loose material and should be compacted within 1 percentage point below to 3 percentage points above the optimum moisture content. If water must be added, it should be uniformly applied and thoroughly mixed into the soil by diskings or scarifying. Each lift of compacted fill should be tested by a representative of the Geotechnical Engineer prior to placement of subsequent lifts. The edge of compacted structural fill should extend at least 10 feet beyond the edge of building prior to sloping.

Crowning the building pad during fill placement, particularly in wet periods, is highly recommended to minimize ponding of water and allow rapid runoff of surface water. Construction traffic should not be allowed on the building pad during wet weather, where practical.

Shallow Foundations

Based on the field data and laboratory test results, the site is suitable to support future industrial developments on shallow foundations bearing at least two (2) feet below the finished grade. Shallow spread and continuous wall footings bearing in the compacted structural fill or in the naturally occurring stiff clay can be designed for maximum allowable bearing pressures of 2,500 and 2,000 pounds per square foot, respectively. Minimum dimensions of 24 inches for spread footings and 18 inches for continuous footings should be used in the design, even if the resulting bearing pressure is less than the allowable bearing pressure, to minimize the possibility of a local bearing failure. The recommended preliminary bearing capacities include a factor of safety of 3.0.

The foundation excavations should be observed by a representative of Stratum Engineering prior to steel or concrete placement to assess that the foundation materials are consistent with the materials discussed in this report. Soft or loose soil zones encountered at the bottom of the footing excavations should be removed to the level of firm, suitable bearing soils or adequately compacted fill as directed by the Geotechnical Engineer.

The footing excavations should be observed and concrete placed as quickly as possible to avoid exposure of the footing bottoms to wetting and drying. Surface run-off water should be drained away from the excavations and not be allowed to pond. If it is required that footing excavations be left open for more than one day, they should be protected to reduce changes in moisture content of the bearing soils.

Settlement

Areal settlement under a building is generally caused by the amount of fill placed, the building footprint and the subsurface soil conditions encountered in the building area. Similarly, footing settlement depends on the footing size as well as the soil conditions below the footing. At the time this report was prepared, the type of structures had not been identified and the amount of fill needed to achieve the design grade was not known. However, based on the subsurface conditions encountered at the site, areal settlement is anticipated to be minimal with the addition of 2 to 3 feet of fill.

Based on the assumed structural loads, foundation settlement will be less than 1 inch provided the footings are designed for the recommended bearing pressure.

Floor Slabs

A slab-on-grade may be supported on compacted low plasticity structural fill. Placement of new structural fill and preparation of the existing subgrade beneath the floor slabs should be prepared as outlined in the "Site Preparation" section of this report. Proofrolling, as discussed earlier in this report, should be accomplished to identify any soft or unstable soils which should be removed from the floor slab areas prior to fill placement and/or floor slab construction.

For design purposes, a Modulus of Subgrade Reaction (k) of 125 pci can be used for the compacted structural fill. This can be increased with the addition of a 4 to 6 inch layer of 610 limestone below the floor slabs. In addition to improving the Modulus of Subgrade Reaction, the aggregate base will evenly distribute the load over the subgrade and provide a better working table during construction.

The floor slabs should have an adequate number of joints to reduce cracks resulting from any differential movement and shrinkage. The floor slabs should not be rigidly connected to columns, walls or foundations. Polyethylene sheeting should be placed on top of the aggregate base to act as a vapor barrier and protect the slabs from potential problems commonly associated with moisture migration through floor slabs in a controlled environment.

Parameters for Pavement Design

Parking areas and drives associated with an industrial park are expected to consist generally of light duty pavement for employee parking as well as heavy duty pavement for large truck staging areas, parking areas and drives.

Based on the field data and laboratory test results, the near surface soil consists of sandy silts or sandy silty clays. Typical California Bearing Ratio (CBR) values for the existing silty subgrade or imported clayey sand structural fill were estimated to be on the order of 4 to 5 corresponding to a Modulus of Subgrade Reaction (k) of 125 to 150 pci which may be used for the design of flexible and rigid pavements, respectively. These values may be used with consideration given to the frequency and magnitude of anticipated traffic loads associated with the type of facilities being constructed.

CONSTRUCTION CONSIDERATIONS

Moisture Sensitive Soils/Weather Related Concerns

The upper silty soils encountered at the site are extremely sensitive to changes in moisture content and may lose significant strength if allowed to become saturated. In addition, soils that become wet may be slow to dry and thus significantly retard the progress of grading and compaction activities. During wet weather periods, increases in the moisture content of the upper soils can cause some reduction in the soil strength and support capabilities. Therefore, it will be advantageous to perform earthwork construction activities during dry weather. The site contractor shall be responsible for maintaining a firm, unyielding and stable subgrade condition. Should the near surface soils become wet, the contractor should be prepared to mitigate these conditions by repeated aeration and exposure to sunlight or by admixture treatment.

REPORT LIMITATIONS

The recommendations submitted in this report are based on the available subsurface information obtained by SE and the preliminary design assumptions typically associated with the type of structures anticipated for the planned development. These recommendations are preliminary and generalized in nature. These recommendations are not considered final, and SE will not be responsible for their use in the design of a specific structure without the opportunity to perform a detailed investigation and determine if changes in the recommendations are required.

The Geotechnical Engineer warrants that the findings, recommendations, specifications, or professional advice contained herein have been made in accordance with generally accepted professional geotechnical engineering practices in the local area. No other warranties are implied or expressed.

Once specific plans for the development are complete, the Geotechnical Engineer should be retained and provided the opportunity to conduct a more thorough geotechnical investigation and analysis utilizing the detailed design plans and specifications for the structures to be constructed.

This report has been prepared for the exclusive use of Darby Holdings, LLC for marketing and planning of the Judy Darby Site in St. Tammany Parish, Louisiana.

APPENDIX



◆ = Proposed Boring Location



BORING LOCATION PLAN

GEOTECHNICAL ENGINEERING SERVICES
 GENERAL SITE CHARACTERIZATION
 JUDY DARBY SITE
 ST. TAMMANY PARISH, LOUISIANA



LOG OF BORING B-1
GENERAL SITE CHARACTERIZATION
JUDY DARBY SITE
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: SOUTH END OF PROPERTY

PROJECT NO.: G15-044

DEPTH, FT.	SOIL TYPE	SAMPLES	DESCRIPTION	N-BLOWS/FT.	UNCONFINED COMPRESSIVE STRENGTH tsf	HAND PENETROMETER tsf	TORVANE tsf	UNIT DRY WEIGHT pcf	MOISTURE CONTENT %	LIQUID LIMIT	PLASTICITY INDEX	% PASSING #200 SIEVE
			8" Silty Sandy Topsoil with organics			4.50			14			
			Very stiff tannish gray Sandy Silty Clay									
			Very stiff tannish gray Lean Clay with sand		5.61	4.50		116	12	44	27	85
5			- with calcareous nodules at 4'			2.00			23			
			Stiff to very stiff gray Lean Clay		1.57	1.75		104	22			
10			- with sand layers at 8'			2.00			25			
			Stiff gray Sandy Fat Clay			1.50			39	53	33	
15												
			- becomes tannish gray at 18'		1.47	2.75		83	37			
20			Boring terminated at 20 feet									
25												
30												
35												
40												
45												
50												

DEPTH OF BORING: 20 Feet
 DATE: 5/13/2015

GROUNDWATER: Encountered at 7 Feet During Drilling



LOG OF BORING B-2
GENERAL SITE CHARACTERIZATION
JUDY DARBY SITE
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: WEST CENTER OF PROPERTY

PROJECT NO.: G15-044

DEPTH, FT.	SOIL TYPE	SAMPLES	DESCRIPTION	N-BLOWS/FT.	UNCONFINED COMPRESSIVE STRENGTH tsf	HAND PENETROMETER tsf	TORVANE tsf	UNIT DRY WEIGHT pcf	MOISTURE CONTENT %	LIQUID LIMIT	PLASTICITY INDEX	% PASSING #200 SIEVE
0			8" Silty Sandy Topsoil with organics			2.50			24			
0			Very stiff gray Silty Sandy Clay									
0			Stiff gray Lean Clay		0.85	1.75		98	29			
5			- becomes tannish gray at 4'			1.50			22			
5			- very stiff at 6'		1.39	3.25		109	23	45	30	91
10			- with sand layers and silt seams, 8' to 17'			2.25			27			
15					0.87	2.25		103	26			
20			Very stiff olive green Fat Clay			3.00			42			
25					0.40	2.25		78	44			
30			Stiff tannish gray Lean Clay			2.00			22			
35			- stiff with silt seams and sand lenses at 33'		0.87	1.75		105	23	31	16	
40						2.75			24			
45			- slickensided at 43'		1.44	2.50		87	34			
50			Stiff tannish gray Sandy Lean Clay			1.50			24			

DEPTH OF BORING: 70 Feet
 DATE: 5/13/2015

GROUNDWATER: Encountered at 3 1/2 Feet During Drilling



LOG OF BORING B-2 (continued)
GENERAL SITE CHARACTERIZATION
JUDY DARBY SITE
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: WEST CENTER OF PROPERTY

PROJECT NO.: G15-044

DEPTH, FT.	SOIL TYPE	SAMPLES	DESCRIPTION	N-BLOWS/FT.	UNCONFINED COMPRESSIVE STRENGTH tsf	HAND PENETROMETER tsf	TORVANE tsf	UNIT DRY WEIGHT pcf	MOISTURE CONTENT %	LIQUID LIMIT	PLASTICITY INDEX	% PASSING #200 SIEVE
55	[Diagonal Hatching]	[Black Box]	Stiff tannish gray Sandy Lean Clay		0.57	1.75		102	23	31	17	57
60			Very stiff tannish gray Fat Clay - slickensided at 58'			3.50			32			
65			- becomes olive green at 63'		0.77	2.50		80	41			
70	[Diagonal Hatching]	[Black Box]	- becomes bluish gray at 68'			2.75			41			
75			Boring terminated at 70 feet									
80												
85												
90												
95												
100												

DEPTH OF BORING: 70 feet
 DATE: 5/13/2015



LOG OF BORING B-3
GENERAL SITE CHARACTERIZATION
JUDY DARBY SITE
ST. TAMMANY PARISH, LOUISIANA

TYPE OF BORING: WET ROTARY

LOCATION: NORTH END OF PROPERTY

PROJECT NO.: G15-044

DEPTH, FT.	SOIL TYPE	SAMPLES	DESCRIPTION	N-BLOWS/FT.	UNCONFINED COMPRESSIVE STRENGTH tsf	HAND PENETROMETER tsf	TORVANE tsf	UNIT DRY WEIGHT pcf	MOISTURE CONTENT %	LIQUID LIMIT	PLASTICITY INDEX	% PASSING #200 SIEVE
			8" Silty Sandy Topsoil with organics			4.00			19	23	2	
			Dense gray Sandy Silt									
			Very stiff tannish gray Sandy Lean Clay		1.68	2.75		110	20			
5			Very stiff tannish gray Lean Clay with sand and silt seams			2.50			21			
					1.53	2.50		101	25			
10						1.50			24	39	23	84
			Very stiff tannish gray Fat Clay									
15					1.14	2.25		86	37			
20						2.00			41			
			Boring terminated at 20 feet									
25												
30												
35												
40												
45												
50												

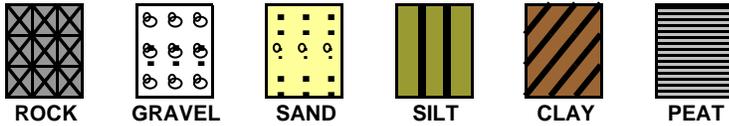
DEPTH OF BORING: 20 Feet
 DATE: 5/13/2015

GROUNDWATER: Encountered at 3 1/2 Feet During Drilling

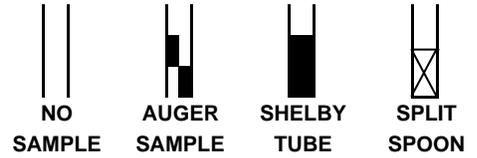


KEY TO TERMS AND SYMBOLS USED ON LOGS

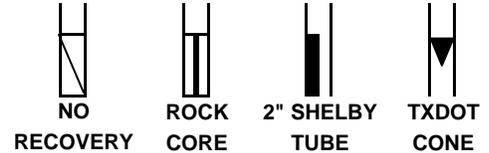
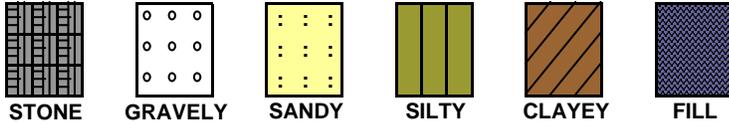
SOIL TYPE



SAMPLER TYPE



MODIFIERS



UNIFIED SOIL CLASSIFICATION SYSTEM - ASTM D 2487 (1980)

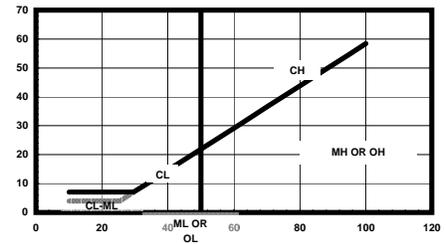
MAJOR DIVISIONS			LETTER SYMBOL	TYPICAL DESCRIPTIONS	
COARSE GRAINED SOILS	GRAVEL & GRAVELLY SOILS	CLEAN GRAVEL (LITTLE OR NO FINES)	GW	WELL GRADED GRAVEL, GRAVEL-SAND MIXTURES WITH LITTLE OR NO FINES	
		LESS THAN 50% PASSING NO. 4 SIEVE	GP	POORLY GRADED GRAVEL, GRAVEL-SAND MIXTURES WITH LITTLE OR NO FINES	
	50% PASSING NO. 4 SIEVE	W/ APPRECIABLE FINES	CLEAN SANDS (LITTLE FINES)	GM	SILTY GRAVEL, GRAVEL-SAND-SILT MIXTURES
			SANDS WITH APPRECIABLE FINES	GC	CLAYEY GRAVELS, GRAVEL-SAND-CLAY MIXTURES
	50% PASSING NO. 200 SIEVE	SANDS WITH LITTLE FINES	CLEAN SANDS (LITTLE FINES)	SW	WELL GRADED SAND, GRAVELY SAND (LITTLE FINES)
			SANDS WITH LITTLE FINES	SP	POORLY GRADED SANDS, GRAVELY SAND (L-FINES)
	50% PASSING NO. 200 SIEVE	SANDS WITH APPRECIABLE FINES	SANDS WITH LITTLE FINES	SM	SILTY SANDS, SAND-SILT MIXTURES
			SANDS WITH APPRECIABLE FINES	SC	CLAYEY SANDS, SAND-CLAY MIXTURES
	FINE GRAINED SOILS	SILTS AND CLAYS	LIQUID LIMIT LESS THAN 50	ML	INORGANIC SILTS & VERY FINE SANDS, ROCK FLOUR SILTY OR CLAYEY FINE SANDS OR CLAYEY SILT W/ LOW PI
			LIQUID LIMIT LESS THAN 50	CL	INORGANIC CLAY OF LOW TO MEDIUM PI LEAN CLAY GRAVELY CLAYS, SANDY CLAYS, SILTY CLAYS
LIQUID LIMIT LESS THAN 50			OL	ORGANIC SILTS & ORGANIC SILTY CLAYS OF LOW PI	
SILTS AND CLAYS		LIQUID LIMIT GREATER THAN 50	MH	INORGANIC SILTS, MICACEOUS OR DIATOMACEOUS FINE SANDY OR SILTY SOILS, ELASTIC SILTS	
	LIQUID LIMIT GREATER THAN 50	CH	INORGANIC CLAYS OF HIGH PLASTICITY FAT CLAYS		
	LIQUID LIMIT GREATER THAN 50	OH	ORGANIC CLAYS OF MED TO HIGH PI, ORGANIC SILT		
HIGHLY ORGANIC SOIL			PT	PEAT AND OTHER HIGHLY ORGANIC SOILS	
UNCLASSIFIED FILL MATERIALS			ARTIFICIALLY DEPOSITED AND OTHER UNCLASSIFIED SOILS AND MAN-MADE SOIL MIXTURES		

CONSISTENCY OF COHESIVE SOILS

CONSISTENCY	SHEAR STRENGTH IN TONS/FT ²
VERY SOFT	0. TO 0.125
SOFT	0.125 TO 0.25
FIRM	0.25 TO 0.5
STIFF	0.5 TO 1.0
VERY STIFF	1.0 TO 2.0
HARD	> 2.0 OR 2.0+

RELATIVE DENSITY - GRANULAR SOILS

CONSISTENCY	N-VALUE (BLOWS/FOOT)
VERY LOOSE	0-4
LOOSE	4-9
MEDIUM DENSE	10-29
DENSE	30-49
VERY DENSE	> 50 OR 50+



ABBREVIATIONS

HP - HAND PENETROMETER UC - UNCONFINED COMPRESSION TEST
 TV - TORVANE UU - UNCONSOLIDATED UNDRAINED TRIAXIAL
 MV - MINIATURE VANE CU - CONSOLIDATED UNDRAINED

NOTE: PLOT INDICATES SHEAR STRENGTH AS OBTAINED BY ABOVE TESTS

▼ DELAYED GROUNDWATER LVL
 ▽ LEVEL GROUNDWATER ENCOUNTERED

CLASSIFICATION OF GRANULAR SOILS

U.S. STANDARD SIEVE SIZE(S)

BOUL- -DERS	6"	3"	3/4"	4	10	40	200	SILT	CLAY
	COBBLES	GRAVEL		SAND					
		COARSE	FINE	COARSE	MEDIUM	FINE			
	152	76.2	19.1	4.76	2.0	0.42	0.075		0.002
				GRAIN SIZE IN MM					