

Exhibit AA.

Henderson I-10 Site

Preliminary Geotechnical Engineering Report





Henderson I-10 Site Preliminary Geotechnical Engineering Report

ECS Southeast, LLC

Preliminary Geotechnical Site Characterization Report
Henderson I-10 Site

Par Rd. 40
Breaux Bridge, St. Martin Parish, Louisiana

ECS Project Number 65-1964

February 16, 2026





ECS SOUTHEAST, LLC

Geotechnical • Construction Materials • Environmental • Facilities

February 16, 2026

Ms. Megan Duhon
One Acadiana
523 Jefferson Street
Lafayette, LA 70501

ECS Project No. 65-1964

Reference: Preliminary Geotechnical Site Characterization Report
Henderson I-10 Site
Par Rd. 40
Breaux Bridge, St. Martin Parish, Louisiana

Dear Ms. Duhon:

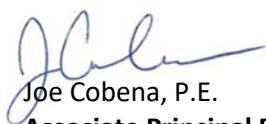
ECS Southeast, LLC (ECS) has completed the subsurface exploration, laboratory testing, and preliminary geotechnical engineering analyses for the Henderson I-10 Site in St. Martin Parish, LA. Our services were performed in general accordance with our Proposal No. 65-3140-P dated July 7, 2025. **This report is not a comprehensive geotechnical engineering report but is solely intended to address specific preliminary issues posed in a June 26, 2025, *Solicitation for Geotechnical Engineering Studies* document from Mr. Elliott Boudreaux of CSRS relative to this site. Additional borings and testing will be required prior to development of the site.** This report presents our understanding of the geotechnical aspects of the project along with the results of the field exploration and laboratory testing conducted. The report also contains our findings and preliminary recommendations for design and construction.

It has been our pleasure to be of service to One Acadiana during the design phase of this project. We would appreciate the opportunity to remain involved during the continuation of the design phase, and we would like to provide our services during construction phase operations as well to verify the assumptions of subsurface conditions made for this report. Should you have any questions concerning the information contained in this report, or if we can be of further assistance to you, please contact us.

Respectfully,
ECS SOUTHEAST, LLC


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"ONE FIRM. ONE MISSION."

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EXECUTIVE SUMMARY

The following summarizes the main findings of the exploration, particularly those that may have a cost impact on the planned development. Further, our preliminary foundation recommendations are summarized. Information gleaned from the Executive Summary should not be utilized in lieu of reading the entire geotechnical report.

PROJECT UNDERSTANDING:

- Structure Information: Industrial development, 100,000 square foot building
- Considerations: Fill heights, settlement

SUBSURFACE CONDITIONS:

- Surface Material: Clay headlands or crop rows with sugar cane
- Probable Fill: No fill material encountered
- Natural Material: Gray and Brown Lean Clay (CL), Gray Fat Clay (CH), Gray Silt (ML), Organics
- Swell Potential (PVR): Low to Moderate, expansive clays encountered in the upper 8 feet above the groundwater table
- Groundwater: Groundwater was encountered at depths of approximately 6 to 9 feet below existing grade during drilling operations

PRELIMINARY DESIGN & CONSTRUCTION RECOMMENDATIONS:

- Shallow Foundations
 - Spread Footings: 2,000 psf Allowable Bearing Pressure
- Deep Foundations
 - 14" Square Pre-Cast Concrete Pile

1.0 INTRODUCTION

The purpose of this study was to conduct a preliminary geotechnical characterization for the property, located off Par Rd. 40 in Breaux Bridge, St. Martin Parish, Louisiana, in order to generally characterize the site's soil, rock, and groundwater conditions to evaluate geotechnical concerns observed at the site. This document specifically addresses preliminary design issues posed in the June 26, 2025, *Solicitation for Geotechnical Engineering Studies* document from Mr. Elliott Boudreaux of CSRS.

Our services were provided in accordance with our Proposal No. 65-3140-P dated July 7, 2025, and authorized by Ms. Megan Duhon with One Acadiana on October 29, 2025.

The preliminary recommendations developed for this report are based on project information provided by the client. This report contains the results of our subsurface exploration and geotechnical laboratory testing program, site characterization, engineering analyses, and preliminary recommendations.

To obtain the necessary geotechnical information required for preliminary evaluation of subsurface soil conditions, three (3) soil test borings were drilled in total. One (1) test boring extended to a depth of 100 feet below existing grades, one (1) test boring extended to a depth of 50 feet, and one (1) test boring extended to a depth of 30 feet below existing site grades. A laboratory testing program was also implemented to characterize the physical and geotechnical engineering properties of the subsurface soils.

The report includes the following items.

- A brief review and description of our field and laboratory test procedures and the results of testing conducted.
- A review of surface topographical features and site conditions.
- A review of subsurface soil stratigraphy with pertinent available physical properties.
- Soil boring logs.
- Preliminary recommendations for site preparation.
- Preliminary recommendations for foundation types.

2.0 PROJECT INFORMATION

2.1 PROJECT LOCATION/CURRENT SITE USE/PAST SITE USE

The project property is located off Par Rd. 40, north of I-10 in Breaux Bridge, St. Martin Parish, Louisiana. The location is shown in Figure 2.1.1.



FIGURE 2.1.1: General Site Location Outlined in Red

The subject site is approximately 146± acres of agricultural land with sugar cane fields located in a commercial and residential setting. Historical imagery obtained from Google Earth© shows the site has remained in the same general condition since 1998. The topography of the site is relatively flat with surface elevations ranging from +18 feet to +21 feet MSL. The elevations and topographic variations were estimated using Google Earth©.

2.2 PROPOSED CONSTRUCTION

ECS understands that the Louisiana Economic Development (LED) Site Certification requires preliminary confirmation that the site is compatible with industrial development and that it could support the construction of a 'typical' manufacturing building encompassing 100,000 square feet and associated on-site roadways and infrastructure. Detailed loading information was not provided to ECS at the time of this report. Soil augmentation that may be required for the construction of the foundations, buildings and roadways is addressed in this report.

3.0 FIELD EXPLORATION AND LABORATORY TESTING

The field exploration was planned with the objective of characterizing the project site in general geotechnical and geological terms and to evaluate subsequent field and laboratory data to assist in the determination of geotechnical recommendations consistent with the criteria specified in the *Solicitation for Geotechnical Engineering Studies* document provided by CSRS.

The subsurface conditions were explored by performing a total of three (3) soil test borings. One (1) test boring extended to a depth of 100 feet below existing site grades, one (1) test boring extended to a depth of 50 feet, and one (1) test boring extended to a depth of 30 feet below existing site grades.

An ATV-mounted rig was utilized to drill the borings with dry auger and wet rotary techniques. The subsurface exploration was completed under the general supervision of an ECS representative.

The boring locations were selected by ECS based on the site plan provided by the client and in coordination with One Acadiana and identified in the field by ECS personnel using the supplied diagram and handheld GPS unit. The approximate as-drilled boring locations are shown on the Boring Location Diagram in Appendix A. The approximate ground surface elevations noted in this report were obtained from Google Earth®.

3.1 REGIONAL/SITE GEOLOGY

The subsurface conditions encountered were generally consistent with published geological mapping. The Henderson I-10 Site is located in south Louisiana and consists of a geologic unit known as Natural Levees. Soils found in the Natural Levees are typically made up of gray and brown silt, silty clay and some very fine sand. Natural Levees were formed in the Holocene age and lie in a physiographic region known as the Mississippi River Alluvial Plain.

3.2 SUBSURFACE CHARACTERIZATION

Representative soil samples were obtained by means of Standard Penetration Test (SPT) procedures in accordance with ASTM Specifications D-1586 in granular soils and by means of Shelby tube sampling procedures in accordance with ASTM Specifications D-1587 in cohesive soils. SPT sampling is performed by driving a split-barrel sampler into the soil in 1.5-foot intervals with a 140-lb hammer and measures the resistance of the soil to penetration of the 2-inch diameter sampler. In the Shelby tube sampling procedure, a thin walled, steel, seamless tube with sharp cutting edges is pushed hydraulically into the soil, and a relatively undisturbed sample is obtained. The following table provides the generalized soil strata encountered.

Approximate Depth (ft)	Stratum No.	Soil Description ⁽²⁾
0.0 – 4.0 ft	I	LEAN CLAY (CL) , Stiff to Very Stiff, Tan, Gray, and Brown
4.0 – 13.0 ft	II	LEAN CLAY (CL) , Soft to Firm, Gray
13.0 – 68.0 ft	III	FAT CLAY (CH) , Firm to Stiff, Gray
68.0 – 73.0 ft	IV	SILT , Very Soft, Gray
73.0 – 83.0 ft	V	LEAN CLAY (CL) , Firm to Stiff, Gray
83.0 – 100.0 ft	VI	FAT CLAY (CH) , Stiff, Gray

1 Please note that the ground surface elevations were or were not surveyed by a licensed surveyor; these elevations are approximate based on Google-Earth®.

2 Soil descriptions show approximate strata to 100'. Strata in the borings vary, please see attached boring logs in Appendix B.

Please refer to the attached boring logs and laboratory data summary for this field exploration for a more detailed description of the subsurface conditions encountered in the borings as the stratification descriptions above are generalized for presentation purposes.

3.3 GROUNDWATER OBSERVATIONS

Groundwater was encountered at depths of approximately 6 to 9 feet below existing grades during drilling operations.

The highest groundwater observations are normally encountered in the late winter or early spring or following seasonal heavy rainfall events. Fluctuation in the location of the long-term water table may occur as a result of changes in precipitation, evaporation, surface water runoff and other factors not immediately apparent at the time of his investigation. Therefore, the groundwater conditions at this site are expected to be significantly influenced by surface water runoff and rainfall.

3.4 LABORATORY TESTING

The laboratory testing was performed by ECS on selected samples obtained during our field exploration operations. Classification and index property tests were performed on representative soil samples obtained from the test borings in order to aid in classifying soils according to the Unified Soil Classification System and to quantify and correlate engineering properties. The soil samples were tested for Moisture Content (ASTM D2216), Atterberg Limits (ASTM D4318), Percent Passing the No. 200 Sieve (ASTM D1140), and Unconfined Compression (ASTM D2166).

An experienced geotechnical professional visually classified each soil sample from the test borings on the basis of texture and plasticity in accordance with the Unified Soil Classification System (USCS) and ASTM D-2487 (Standard Practice for Classification for Engineering Purposes). After classification, a geotechnical engineer grouped the various soil types into the major zones noted on the boring logs in Appendix B. The group symbols for each soil type are indicated in parentheses following the soil descriptions on the boring logs. The stratification lines designating the interfaces between earth materials on the boring logs are approximate; in situ, the transitions may be gradual.

The soil samples will be retained in our laboratory for a period of 60 days, after which, they will be discarded unless other instructions are received as to their disposition.

4.0 PRELIMINARY GEOTECHNICAL RECOMMENDATIONS

The following preliminary recommendations have been developed based on the previously described project characteristics and subsurface conditions. These recommendations are preliminary in nature and are for planning purposes only as they are based on limited geotechnical exploration data. The preliminary recommendations herein should not be used for design or construction. Final design and construction recommendations for planned structures will require a thorough design-level geotechnical investigation and engineering analysis.

The proposed site is generally compatible with industrial development depending on the type and anticipated loads of the proposed structures. Due to the site's variability in elevation, we anticipate that more than 2 feet of fill may be required to reach the finished floor elevation of an industrial manufacturing building, and sizes of footings and allowable bearing pressures may vary based on fill heights and site grading activities. The following sections of this document present our preliminary recommendations regarding the proposed site.

4.1 SHALLOW FOUNDATIONS

Provided that subgrades and engineered fills are prepared properly, a typical lightly loaded industrial structure should be able to be supported by conventional shallow spread footings assuming there is no more than 2 feet of fill placed above existing elevations. A net allowable soil bearing capacity on the order of 2,000 psf, with 1-Inch or less of settlement, may be used for preliminary planning and budgeting purposes for footings bearing on stiff in-situ clay or compacted engineered fill. This is based on applying a factor of safety of 3. Footings should extend at least 24 inches below grade. The table below provides estimated size for square footing dimensions based on assumed column loads as required by the CSRS document:

PRELIMINARY ESTIMATED SQUARE SHALLOW FOOTING SIZE		
Assumed Column Load (Kips)	Spread Footing Plan Dimensions	
	Depth (ft)	Length/Width (ft)
8	2	2
12.5	2	2.5
18	2	3

These preliminary design parameters assume that positive drainage will be provided away from structures and with no excessive wetting or drying of soils adjacent to the foundations. Greater potential movements could occur with extreme wetting or drying of the soil due to ponding of water, plumbing leaks, lack of irrigation or improper landscaping.

The net allowable soil bearing pressure refers to the pressure which may be transmitted to the foundation bearing soils in excess of the final minimum surrounding overburden pressure. A licensed Professional Engineer should evaluate the final footing and/or grade beam elevation to verify that the bearing soils are capable of supporting the recommended net allowable bearing pressure and are adequate for foundation construction.

4.2 DEEP FOUNDATIONS

Typical considerations are provided below for deep foundations should a more heavily loaded structure be proposed for the subject site. *It should be reemphasized that these values provided should be used for planning and budgeting purposes and should be reevaluated once a specific design is developed for the site.*

The preliminary allowable capacities for 14-inch square precast prestressed concrete (PPC) piles are presented, for a range of pile lengths, in the following table for use in feasibility studies, planning, and cost estimating purposes per the CSRS provided document:

PRELIMINARY ESTIMATED ALLOWABLE SINGLE PILE CAPACITIES (KIPS)		
Pile Length (ft)	14-inch Square PPC Pile	
	Compression (kips)	Tension (kips)
30	38	22
40	55	33
50	72	45
60	90	56
70	102	67
80	118	76
90	150	94

The estimated allowable pile capacities, in the above table, include a factor of safety of 2.0 in compression and 3.0 in tension. A load test program, compression, tension and lateral, would be required for final design capacities. If a load test program is not performed, ECS recommends using an increased factor of safety of 3.0 for compression to determine the allowable capacities. The recommended pile lengths are referenced from the existing ground surface at the time of drilling. The allowable capacity estimates provided in the table are based on field and laboratory testing and assume proper design and installation. Please note that these estimated capacities do not account for negative skin friction effects, heave, swell pressures, or fill placement that may reduce total capacity.

5.0 SITE CONSTRUCTION RECOMMENDATIONS

5.1 SITE PREPARATION

In a dry and undisturbed state, the near-surface soils should provide subgrade support for engineered fill placement and construction operations. However, when wet, this soil will degrade quickly with disturbance from contractor operations. Chemical stabilization of the in-situ soils with lime, lime kiln dust (LKD), or Portland cement may be necessary depending on seasonal conditions. Therefore, good site drainage should be maintained during earthwork operations, which can help maintain the integrity of the soil.

Note: Soils associated with former sugar cane fields are generally not suitable for cement treatment.

The surface of the site should be kept properly graded to promote drainage of the surface water away from the proposed building areas during the construction phase. We recommend that an attempt be made to enhance the natural drainage without interrupting its pattern.

The soils encountered at the site are moisture and disturbance sensitive and contain fines which are considered moderately erodible. Therefore, the contractor should carefully plan his operation to minimize exposure of the subgrade to weather and construction equipment traffic and provide and maintain good site drainage during earthwork operations to help maintain the integrity of the surficial soils. All erosion and sedimentation shall be controlled in accordance with sound engineering practice and current jurisdictional requirements.

In preparing the site for construction, all loose, poorly compacted existing soils, vegetation, organic soil, existing pavements, foundations or utilities, existing fill material, or other inadequate materials should be removed from all proposed building and paving areas, and areas receiving new fill.

6.0 REPORT LIMITATIONS AND CLOSING

ECS has prepared this report of findings, evaluations, and *preliminary* recommendations to generally characterize the sites soil and groundwater conditions to evaluate whether geotechnical concerns were observed at the site.

The preliminary recommendations provided in this report are based on the data obtained from the limited field exploration and laboratory testing at the specified boring locations for the purpose of a general site characterization. The recommendations are not intended for use in final design or construction. Final design and construction recommendations for any structure proposed on the site will require a more detailed investigation and engineering analysis.

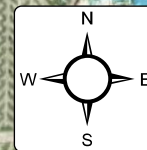
The description of the proposed site is based on information provided to ECS by Mr. Elliott Boudreaux of CSRS, Inc. If any of this information is inaccurate, either due to our interpretation of the documents provided or site changes that may occur later, ECS should be contacted immediately so we can review the recommendations provided considering the changes and provide additional or alternate recommendations as may be required to reflect the proposed site.

Appendix A - Drawings and Reports

Site Location Diagram

Boring Location Diagram(s)

Service Layer Credits: World Boundaries and Place Names Sources: Esri, HERE, Garmin, FAO, NOAA, USGS, ©



SITE LOCATION DIAGRAM

Henderson I-10 Site

Par Rd 40, St. Martin Parish, Louisiana
One Acadiana

ENGINEER
JCC3

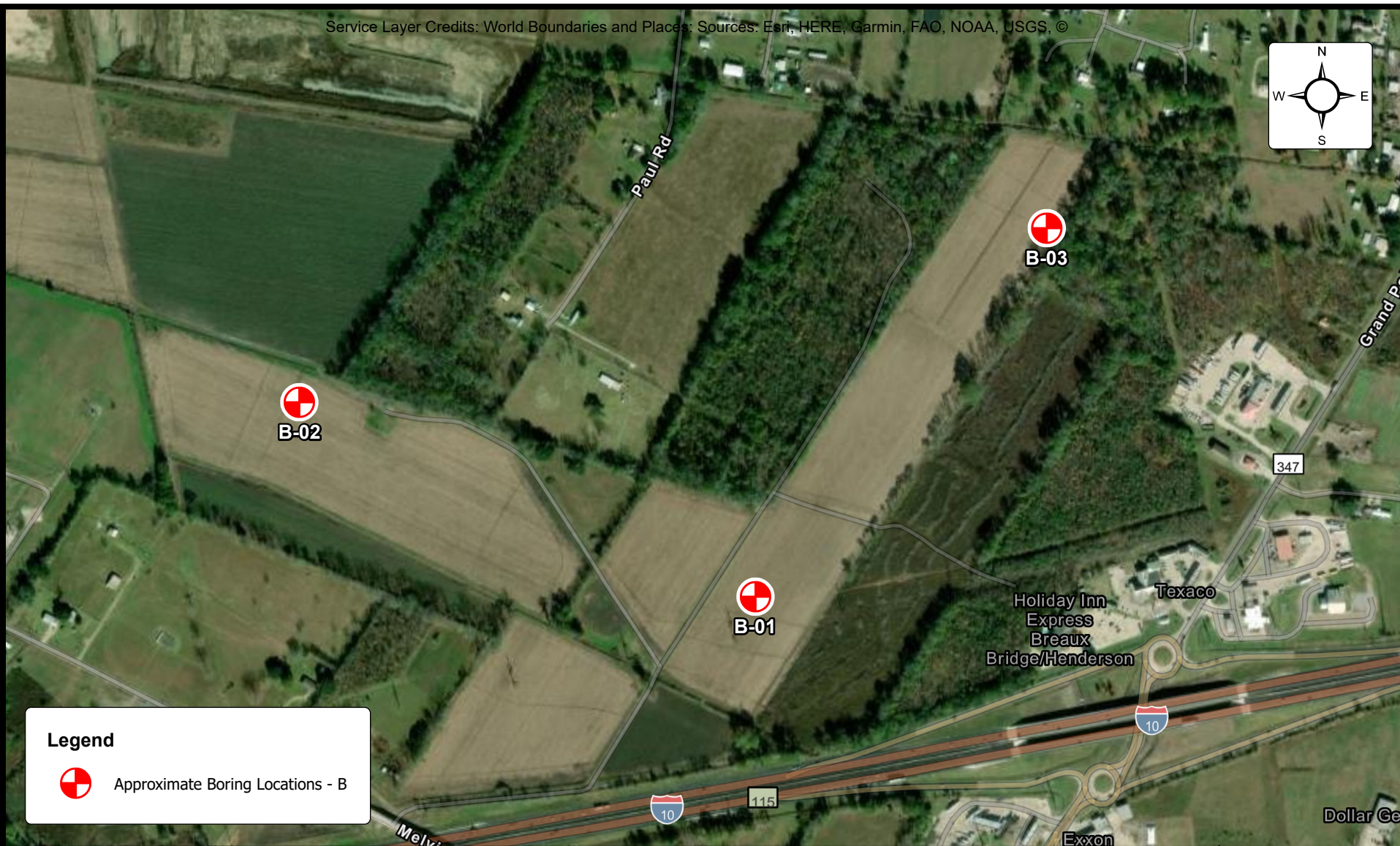
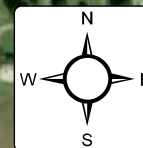
SCALE
1" = 900'

PROJECT NO.
65:1964

SHEET

DATE
2/12/2026

Service Layer Credits: World Boundaries and Places: Sources: Esri, HERE, Garmin, FAO, NOAA, USGS, ©



Legend



Approximate Boring Locations - B



BORING LOCATION DIAGRAM

Henderson I-10 Site

Par Rd 40, St. Martin Parish, Louisiana
One Acadiana

ENGINEER
JCC3

SCALE
1" = 600'

PROJECT NO.
65:1964

SHEET

DATE
2/12/2026

Appendix B - Field Operations

Reference Notes

Boring Logs



REFERENCE NOTES FOR BORING LOGS

MATERIAL^{1,2}

	ASPHALT
	CONCRETE
	GRAVEL
	TOPSOIL
	VOID
	BRICK
	AGGREGATE BASE COURSE
	GW WELL-GRADED GRAVEL gravel-sand mixtures, little or no fines
	GP POORLY-GRADED GRAVEL gravel-sand mixtures, little or no fines
	GM SILTY GRAVEL gravel-sand-silt mixtures
	GC CLAYEY GRAVEL gravel-sand-clay mixtures
	SW WELL-GRADED SAND gravelly sand, little or no fines
	SP POORLY-GRADED SAND gravelly sand, little or no fines
	SM SILTY SAND sand-silt mixtures
	SC CLAYEY SAND sand-clay mixtures
	ML SILT non-plastic to medium plasticity
	MH ELASTIC SILT high plasticity
	CL LEAN CLAY low to medium plasticity
	CH FAT CLAY high plasticity
	OL ORGANIC SILT or CLAY non-plastic to low plasticity
	OH ORGANIC SILT or CLAY high plasticity
	PT PEAT highly organic soils

DRILLING SAMPLING SYMBOLS & ABBREVIATIONS

SS	Split Spoon Sampler	PM	Pressuremeter Test
ST	Shelby Tube Sampler	RD	Rock Bit Drilling
WS	Wash Sample	RC	Rock Core, NX, BX, AX
BS	Bulk Sample of Cuttings	REC	Rock Sample Recovery %
PA	Power Auger (no sample)	RQD	Rock Quality Designation %
HSA	Hollow Stem Auger		

PARTICLE SIZE IDENTIFICATION

DESIGNATION	PARTICLE SIZES
Boulders	12 inches (300 mm) or larger
Cobbles	3 inches to 12 inches (75 mm to 300 mm)
Gravel: Coarse	¾ inch to 3 inches (19 mm to 75 mm)
Fine	4.75 mm to 19 mm (No. 4 sieve to ¾ inch)
Sand: Coarse	2.00 mm to 4.75 mm (No. 10 to No. 4 sieve)
Medium	0.425 mm to 2.00 mm (No. 40 to No. 10 sieve)
Fine	0.074 mm to 0.425 mm (No. 200 to No. 40 sieve)
Silt & Clay ("Fines")	<0.074 mm (smaller than a No. 200 sieve)

COHESIVE SILTS & CLAYS

UNCONFINED COMPRESSION STRENGTH, QP ⁴	SPT ⁵ (BPF)	CONSISTENCY ⁷ (COHESIVE)
<0.25	<2	Very Soft
0.25 - <0.50	3 - 4	Soft
0.50 - <1.00	5 - 8	Firm
1.00 - <2.00	9 - 15	Stiff
2.00 - <4.00	16 - 30	Very Stiff
4.00 - 8.00	31 - 50	Hard
>8.00	>50	Very Hard

RELATIVE AMOUNT ⁷	COARSE GRAINED (%) ⁸	FINE GRAINED (%) ⁸
Trace	≤5	≤5
With	10 - 20	10 - 25
Adjective (ex: "Silty")	25 - 45	30 - 45

GRAVELS, SANDS & NON-COHESIVE SILTS

SPT ⁵	DENSITY
<5	Very Loose
5 - 10	Loose
11 - 30	Medium Dense
31 - 50	Dense
>50	Very Dense

WATER LEVELS⁶

	WL (First Encountered)
	WL (Completion)
	WL (Seasonal High Water)
	WL (Stabilized)

FILL AND ROCK

FILL	POSSIBLE FILL	PROBABLE FILL	ROCK

¹Classifications and symbols per ASTM D 2488-17 (Visual-Manual Procedure) unless noted otherwise.

²To be consistent with general practice, "POORLY GRADED" has been removed from GP, GP-GM, GP-GC, SP, SP-SM, SP-SC soil types on the boring logs.

³Non-ASTM designations are included in soil descriptions and symbols along with ASTM symbol [Ex: (SM-FILL)].

⁴Typically estimated via pocket penetrometer or Torvane shear test and expressed in tons per square foot (tsf).

⁵Standard Penetration Test (SPT) refers to the number of hammer blows (blow count) of a 140 lb. hammer falling 30 inches on a 2 inch OD split spoon sampler required to drive the sampler 12 inches (ASTM D 1586). "N-value" is another term for "blow count" and is expressed in blows per foot (bpf). SPT correlations per 7.4.2 Method B and need to be corrected if using an auto hammer.

⁶The water levels are those levels actually measured in the borehole at the times indicated by the symbol. The measurements are relatively reliable when augering, without adding fluids, in granular soils. In clay and cohesive silts, the determination of water levels may require several days for the water level to stabilize. In such cases, additional methods of measurement are generally employed.

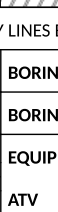
⁷Minor deviation from ASTM D 2488-17 Note 14.

⁸Percentages are estimated to the nearest 5% per ASTM D 2488-17.

CLIENT: CSRS, Inc.				PROJECT NO.: 65:1964		BORING NO.: B-01		SHEET: 1 OF 4									
PROJECT NAME: Henderson I-10 Site				DRILLER/CONTRACTOR: ECS													
SITE LOCATION: Par Rd 40, Breaux Bridge, Louisiana, 70517								LOSS OF CIRCULATION									
LATITUDE: 30.317128		LONGITUDE: -91.836139		STRUCTURE:		SURFACE ELEVATION: 19		BOTTOM OF CASING									
DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DISTANCE	SAMPLE RECOVERY (IN)	DESCRIPTION OF MATERIAL	STRATIGRAPHY	ELEVATION (FT)	WATER LEVELS	BLOWS/6" (TCP/MC/SPT N-VALUE)	Rec % 0 20 40 60 80 100 RQD % 0 10 20 30 40 50 SPT 	LIQUID LIMIT	PLASTICITY INDEX	MOISTUE CONTENT (%)	QP (TSF)	FINES CONTENT	DRY UNIT WEIGHT (PCF)	COMPRESSIVE STR. (TSF)
5	S-1	ST	24	15	(CL) LEAN CLAY - gray, stiff to very stiff.		15							22.9	4.5		
	S-2	ST	24	10										22.9	2.0		
	S-3	ST	24	10										28.0	0.75		
	S-4	ST	24	14										29.0	0.75		
	S-5	ST	24	11										36.3	0.75		
15	S-6	ST	24	14	(CH) FAT CLAY - gray, firm to stiff.		5						44.3	0.5			
20	S-7	ST	24	11			0						41.7	0.75			
25	S-8	ST	24	11			-5						44.1	1.5			
	S-9	ST	24	10			-10						34.5	1.0			
THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL																	
 WL (First Encountered):				9 FT		BORING STARTED:				11/24/2025		CAVE IN DEPTH:				Not Observed	
 WL (Completion):						BORING COMPLETED:				11/24/2025		HAMMER TYPE:				Automatic	
 WL (Seasonal High Water):						EQUIPMENT:		LOGGED BY:		DRILLING METHOD: Solid Auger (0'-10'), Wet Rotary (10'-100')							
 WL (Stabilized):						ATV		TC									
GEOTECHNICAL BOREHOLE LOG																	

[illegible]

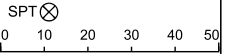
CLIENT: CSRS, Inc.				PROJECT NO.: 65:1964		BORING NO.: B-01		SHEET: 3 OF 4			
PROJECT NAME: Henderson I-10 Site				DRILLER/CONTRACTOR: ECS							
SITE LOCATION: Par Rd 40, Breaux Bridge, Louisiana, 70517								LOSS OF CIRCULATION			
LATITUDE: 30.317128		LONGITUDE: -91.836139		STRUCTURE:		SURFACE ELEVATION: 19		BOTTOM OF CASING			

DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DISTANCE	SAMPLE RECOVERY (IN)	DESCRIPTION OF MATERIAL	STRATIGRAPHY	ELEVATION (FT)	WATER LEVELS	BLOWS/6" (TCP/MC/SPT N-VALUE)	Rec %  RQD % 		LIQUID LIMIT	PLASTICITY INDEX	MOISTURE CONTENT (%)	QP (TSF)	FINES CONTENT	DRY UNIT WEIGHT (PCF)	COMPRESSIVE STR. (TSF)
										SPT 								
65	S-16	ST	24	12	(CH) FAT CLAY - gray, firm to stiff. (continued)		-45							40.9	0.75			
70	S-17	ST	24	16	(ML) SILT - trace sand, gray, very soft.		-50					29	6	37.1	0.25	87.0	81.38	0.23
75	S-18	SS	18	18	(CL) LEAN CLAY - gray, firm to stiff.		-55	4-6-6 (12)	12			34	12	35.6		98.2		
80	S-19	SS	18	18			-60	3-4-4 (8)	8					37.7				
85	S-20	SS	18	12	(CH) FAT CLAY - gray, stiff.		-65	5-5-6 (11)	11					44.3				
	S-21	SS	18	18			-70	3-5-5 (10)	10					47.5				

THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL

 WL (First Encountered): 9 FT		BORING STARTED: 11/24/2025		CAVE IN DEPTH: Not Observed	
 WL (Completion):		BORING COMPLETED: 11/24/2025		HAMMER TYPE: Automatic	
 WL (Seasonal High Water):		EQUIPMENT:		LOGGED BY:	
 WL (Stabilized):		ATV		TC	

GEOTECHNICAL BOREHOLE LOG

CLIENT: CSRS, Inc.					PROJECT NO.: 65:1964			BORING NO.: B-01			SHEET: 4 OF 4														
PROJECT NAME: Henderson I-10 Site					DRILLER/CONTRACTOR: ECS																				
SITE LOCATION: Par Rd 40, Breaux Bridge, Louisiana, 70517										LOSS OF CIRCULATION															
LATITUDE: 30.317128			LONGITUDE: -91.836139			STRUCTURE:			SURFACE ELEVATION: 19			BOTTOM OF CASING													
DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DISTANCE	SAMPLE RECOVERY (IN)	DESCRIPTION OF MATERIAL	STRATIGRAPHY	ELEVATION (FT)	WATER LEVELS	BLOWS/6" (TCP/INC/SPT N-VALUE)	Rec %  RQD % 		LIQUID LIMIT	PLASTICITY INDEX	MOISTUE CONTENT (%)	QP (TSF)	FINES CONTENT	DRY UNIT WEIGHT (PCF)	COMPRESSIVE STR. (TSF)							
									SPT 																
95	S-22	ST	24	18	(CH) FAT CLAY - gray, stiff. (continued)		-75			10		99	66	57.9	1.25		65.82	1.44							
	S-23	ST	24	24					-80												50.1	1.5			
					END OF BORING at 100.0 FT																				
THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL																									
 WL (First Encountered):					9 FT			BORING STARTED:			11/24/2025			CAVE IN DEPTH:			Not Observed								
 WL (Completion):								BORING COMPLETED:			11/24/2025			HAMMER TYPE:			Automatic								
 WL (Seasonal High Water):								EQUIPMENT:		LOGGED BY:		DRILLING METHOD: Solid Auger (0'-10'), Wet Rotary (10'-100')													
 WL (Stabilized):								ATV		TC															
GEOTECHNICAL BOREHOLE LOG																									

CLIENT: CSRS, Inc.						PROJECT NO.: 65:1964			BORING NO.: B-02			SHEET: 1 OF 2																																																																																																																																																																																																																																																																																																																																																																																																																																										
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LATITUDE: 30.31879			LONGITUDE: -91.842469			STRUCTURE:			SURFACE ELEVATION: 20			BOTTOM OF CASING 																																																																																																																																																																																																																																																																																																																																																																																																																																										
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(TSF)</th></tr><tr><th colspan="4">SPT ⊗</th></tr><tr><th>0</th><th>20</th><th>40</th><th>60</th><th>80</th><th>100</th></tr></thead><tbody><tr><td rowspan="5">5</td><td>S-1</td><td>ST</td><td>24</td><td>6</td><td>(CL) LEAN CLAY - gray, stiff.</td><td rowspan="9"></td><td rowspan="9">15</td><td rowspan="9"></td><td rowspan="9"></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>22.9</td><td>1.5</td><td rowspan="9"></td><td rowspan="9"></td></tr><tr><td>S-2</td><td>ST</td><td>24</td><td>10</td><td>(CH) FAT CLAY - gray, firm.</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>68</td><td>48</td><td>30.3</td><td>0.75</td></tr><tr><td>S-3</td><td>ST</td><td>24</td><td>12</td><td>(CL) LEAN CLAY - gray, firm.</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>30.3</td><td>0.5</td></tr><tr><td>S-4</td><td>ST</td><td>24</td><td>13</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>27.4</td><td>0.5</td></tr><tr><td>S-5</td><td>ST</td><td>24</td><td>14</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>34.7</td><td>0.5</td></tr><tr><td rowspan="3">10</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr><tr><td>S-6</td><td>ST</td><td>24</td><td>16</td><td>(CH) FAT CLAY - gray, soft.</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>50</td><td>28</td><td>45.8</td><td>0.25</td></tr><tr><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr><tr><td rowspan="5">15</td><td></td><td></td><td></td><td></td><td></td><td></td><td>5</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr><tr><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr><tr><td>S-7</td><td>ST</td><td>24</td><td>12</td><td>(CH) FAT CLAY - gray, firm to stiff.</td><td></td><td>0</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>1.5</td><td></td></tr><tr><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr><tr><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr><tr><td rowspan="5">20</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr><tr><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr><tr><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr><tr><td>S-8</td><td>ST</td><td>24</td><td>6</td><td></td><td></td><td>-5</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>1.25</td><td></td></tr><tr><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr><tr><td rowspan="4">25</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr><tr><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr><tr><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></tr><tr><td>S-9</td><td>ST</td><td>24</td><td>12</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td>1.5</td><td></td></tr></tbody></table>																DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DISTANCE	SAMPLE RECOVERY (IN)	DESCRIPTION OF MATERIAL	STRATIGRAPHY	ELEVATION (FT)	WATER LEVELS	BLOWS(6" (TCP/MC/SPT N-VALUE)	Rec % ■ RQD % ■				LIQUID LIMIT	PLASTICITY INDEX	MOISTUE CONTENT (%)	QP (TSF)	FINES CONTENT	DRY UNIT WEIGHT (PCF)	COMPRESSIVE STR. (TSF)	SPT ⊗				0	20	40	60	80	100	5	S-1	ST	24	6	(CL) LEAN CLAY - gray, stiff.		15										22.9	1.5			S-2	ST	24	10	(CH) FAT CLAY - gray, firm.								68	48	30.3	0.75	S-3	ST	24	12	(CL) LEAN CLAY - gray, firm.									30.3	0.5	S-4	ST	24	13										27.4	0.5	S-5	ST	24	14										34.7	0.5	10																		S-6	ST	24	16	(CH) FAT CLAY - gray, soft.									50	28	45.8	0.25																		15							5																															S-7	ST	24	12	(CH) FAT CLAY - gray, firm to stiff.		0										1.5																																						20																																																								S-8	ST	24	6			-5										1.25																				25																																																								S-9	ST	24	12													1.5	
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CLIENT: CSRS, Inc.					PROJECT NO.: 65:1964			BORING NO.: B-02			SHEET: 2 OF 2							
PROJECT NAME: Henderson I-10 Site					DRILLER/CONTRACTOR: ECS													
SITE LOCATION: Par Rd 40, Breaux Bridge, Louisiana, 70517										LOSS OF CIRCULATION								
LATITUDE: 30.31879			LONGITUDE: -91.842469			STRUCTURE:			SURFACE ELEVATION: 20			BOTTOM OF CASING						
DEPTH (FT)	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE DISTANCE	SAMPLE RECOVERY (IN)	DESCRIPTION OF MATERIAL	STRATIGRAPHY	ELEVATION (FT)	WATER LEVELS	BLOWS/6" (TCP/INC/SPT N-VALUE)	Rec % 0 20 40 60 80 100 RQD % 0 10 20 30 40 50	LIQUID LIMIT	PLASTICITY INDEX	MOISTURE CONTENT (%)	QP (TSF)	FINES CONTENT	DRY UNIT WEIGHT (PCF)	COMPRESSIVE STR. (TSF)	
35	S-10	ST	24	24	(CH) FAT CLAY - gray, firm to stiff. (continued)		-15				55	33	36.0	0.5			84.23	0.62
40	S-11	SS	18	18	ORGANICS, Firm.		-20		2-2-4 (6)				653.8					
45	S-12	ST	24	6	(CH) FAT CLAY - gray, firm to stiff.		-25							0.5				
	S-13	ST	24	13			-30							1.5				
					END OF BORING at 50.0 FT													
THE STRATIFICATION LINES REPRESENT THE APPROXIMATE BOUNDARY LINES BETWEEN SOIL TYPES. IN-SITU THE TRANSITION MAY BE GRADUAL																		
 WL (First Encountered): 6 FT					BORING STARTED: 11/25/2025					CAVE IN DEPTH: Not Observed								
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 WL (Seasonal High Water):					EQUIPMENT:		LOGGED BY:		DRILLING METHOD: Solid Auger (0'-8'), Wet Rotary (8'-50')									
 WL (Stabilized):					ATV		TC											
GEOTECHNICAL BOREHOLE LOG																		

CLIENT: CSRS, Inc.				PROJECT NO.: 65:1964				BORING NO.: B-03				SHEET: 1 OF 1																																																																																																																																																																																																																																																																																																			
PROJECT NAME: Henderson I-10 Site				DRILLER/CONTRACTOR: ECS																																																																																																																																																																																																																																																																																																											
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LATITUDE: 30.321224				LONGITUDE: -91.832394				STRUCTURE:				SURFACE ELEVATION: 19				BOTTOM OF CASING																																																																																																																																																																																																																																																																																															
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 WL (Seasonal High Water):								EQUIPMENT:				LOGGED BY:				DRILLING METHOD:				Solid Auger (0'-8'), Wet Rotary (8'-30')																																																																																																																																																																																																																																																																																											
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Appendix C - Laboratory Testing

Laboratory Testing Summary

Laboratory Testing Summary

Sample Location	Sample Number	Depth (ft)	^MC (%)	Soil Type	Atterberg Limits			**Percent Passing No. 200 Sieve	Moisture - Density		CBR (%)		#Organic Content (%)
					LL	PL	PI		<Maximum Density (pcf)	<Optimum Moisture (%)	0.1 in.	0.2 in.	
B-01	S-1	0.0-2.0	22.9										
B-01	S-2	2.0-4.0	22.9										
B-01	S-3	4.0-6.0	28.0										
B-01	S-4	6.0-8.0	29.0		35	20	15						
B-01	S-5	8.0-10.0	36.3										
B-01	S-6	13.0-15.0	44.3										
B-01	S-7	18.0-20.0	41.7										
B-01	S-8	23.0-25.0	44.1										
B-01	S-9	28.0-30.0	34.5										
B-01	S-10	33.0-35.0	35.9										

Notes: See test reports for test method, ^ASTM D2216-19, *ASTM D2488, **ASTM D1140-17, #ASTM D2974-20e1 < See test report for D4718 corrected values

Definitions: MC: Moisture Content, Soil Type: USCS (Unified Soil Classification System), LL: Liquid Limit, PL: Plastic Limit, PI: Plasticity Index, CBR: California Bearing Ratio, OC: Organic Content

Project: Henderson I-10 Site
Client: CSRS, Inc.

Project No.: 65:1964
Date Reported: 2/9/2026



Office / Lab

Address

Office Number / Fax

ECS Southeast LLC - Baton Rouge

11211 Industriplex Blvd
Suite 300
Baton Rouge, LA 70809

(225)224-2583
(225)612-7062

Tested by	Checked by	Approved by	Date Received
JMercante	NBurke	NBurke	

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					LL	PL	PI		<Maximum Density (pcf)	<Optimum Moisture (%)	0.1 in.	0.2 in.	
B-01	S-11	38.0-40.0	21.1										
B-01	S-12	43.0-45.0	44.6										
B-01	S-13	48.0-50.0	40.3										
B-01	S-14	53.0-55.0	44.6										
B-01	S-15	58.0-60.0	48.0										
B-01	S-16	63.0-65.0	40.9										
B-01	S-17	68.0-70.0	37.1	ML	29	23	6	87.0					
B-01	S-18	73.5-75.0	35.6	CL	34	22	12	98.2					
B-01	S-19	78.5-80.0	37.7										
B-01	S-20	83.5-85.0	44.3										

Notes: See test reports for test method, ^ASTM D2216-19, *ASTM D2488, **ASTM D1140-17, #ASTM D2974-20e1 < See test report for D4718 corrected values

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B-01	S-21	88.5-90.0	47.5										
B-01	S-22	93.0-95.0	57.9		99	33	66						
B-01	S-23	98.0-100.0	50.1										
B-02	S-1	0.0-2.0	22.9										
B-02	S-2	2.0-4.0	30.3		68	20	48						
B-02	S-3	4.0-6.0	30.3										
B-02	S-4	6.0-8.0	27.4										
B-02	S-5	8.0-10.0	34.7										
B-02	S-6	13.0-15.0	45.8		50	22	28						
B-02	S-10	33.0-35.0	36.0		55	22	33						

Notes: See test reports for test method, ^ASTM D2216-19, *ASTM D2488, **ASTM D1140-17, #ASTM D2974-20e1 < See test report for D4718 corrected values

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B-03	S-1	0.0-2.0	30.1										
B-03	S-2	2.0-4.0	29.2										
B-03	S-3	4.0-6.0	31.5		37	21	16						
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Important Information about This Geotechnical-Engineering Report

Subsurface problems are a principal cause of construction delays, cost overruns, claims, and disputes.

While you cannot eliminate all such risks, you can manage them. The following information is provided to help.

The Geoprofessional Business Association (GBA) has prepared this advisory to help you – assumedly a client representative – interpret and apply this geotechnical-engineering report as effectively as possible. In that way, clients can benefit from a lowered exposure to the subsurface problems that, for decades, have been a principal cause of construction delays, cost overruns, claims, and disputes. If you have questions or want more information about any of the issues discussed below, contact your GBA-member geotechnical engineer. Active involvement in the Geoprofessional Business Association exposes geotechnical engineers to a wide array of risk-confrontation techniques that can be of genuine benefit for everyone involved with a construction project.

Geotechnical-Engineering Services Are Performed for Specific Purposes, Persons, and Projects

Geotechnical engineers structure their services to meet the specific needs of their clients. A geotechnical-engineering study conducted for a given civil engineer will not likely meet the needs of a civil-works constructor or even a different civil engineer. Because each geotechnical-engineering study is unique, each geotechnical-engineering report is unique, prepared *solely* for the client. *Those who rely on a geotechnical-engineering report prepared for a different client can be seriously misled.* No one except authorized client representatives should rely on this geotechnical-engineering report without first conferring with the geotechnical engineer who prepared it. *And no one – not even you – should apply this report for any purpose or project except the one originally contemplated.*

Read this Report in Full

Costly problems have occurred because those relying on a geotechnical-engineering report did not read it *in its entirety*. Do not rely on an executive summary. Do not read selected elements only. *Read this report in full.*

You Need to Inform Your Geotechnical Engineer about Change

Your geotechnical engineer considered unique, project-specific factors when designing the study behind this report and developing the confirmation-dependent recommendations the report conveys. A few typical factors include:

- the client's goals, objectives, budget, schedule, and risk-management preferences;
- the general nature of the structure involved, its size, configuration, and performance criteria;
- the structure's location and orientation on the site; and
- other planned or existing site improvements, such as retaining walls, access roads, parking lots, and underground utilities.

Typical changes that could erode the reliability of this report include those that affect:

- the site's size or shape;
- the function of the proposed structure, as when it's changed from a parking garage to an office building, or from a light-industrial plant to a refrigerated warehouse;
- the elevation, configuration, location, orientation, or weight of the proposed structure;
- the composition of the design team; or
- project ownership.

As a general rule, *always* inform your geotechnical engineer of project changes – even minor ones – and request an assessment of their impact. *The geotechnical engineer who prepared this report cannot accept responsibility or liability for problems that arise because the geotechnical engineer was not informed about developments the engineer otherwise would have considered.*

This Report May Not Be Reliable

Do not rely on this report if your geotechnical engineer prepared it:

- for a different client;
- for a different project;
- for a different site (that may or may not include all or a portion of the original site); or
- before important events occurred at the site or adjacent to it; e.g., man-made events like construction or environmental remediation, or natural events like floods, droughts, earthquakes, or groundwater fluctuations.

Note, too, that it could be unwise to rely on a geotechnical-engineering report whose reliability may have been affected by the passage of time, because of factors like changed subsurface conditions; new or modified codes, standards, or regulations; or new techniques or tools. *If your geotechnical engineer has not indicated an "apply-by" date on the report, ask what it should be, and, in general, if you are the least bit uncertain about the continued reliability of this report, contact your geotechnical engineer before applying it.* A minor amount of additional testing or analysis – if any is required at all – could prevent major problems.

Most of the "Findings" Related in This Report Are Professional Opinions

Before construction begins, geotechnical engineers explore a site's subsurface through various sampling and testing procedures. *Geotechnical engineers can observe actual subsurface conditions only at those specific locations where sampling and testing were performed.* The data derived from that sampling and testing were reviewed by your geotechnical engineer, who then applied professional judgment to form opinions about subsurface conditions throughout the site. Actual sitewide-subsurface conditions may differ – maybe significantly – from those indicated in this report. Confront that risk by retaining your geotechnical engineer to serve on the design team from project start to project finish, so the individual can provide informed guidance quickly, whenever needed.

This Report's Recommendations Are Confirmation-Dependent

The recommendations included in this report – including any options or alternatives – are confirmation-dependent. In other words, *they are not final*, because the geotechnical engineer who developed them relied heavily on judgment and opinion to do so. Your geotechnical engineer can finalize the recommendations *only after observing actual subsurface conditions* revealed during construction. If through observation your geotechnical engineer confirms that the conditions assumed to exist actually do exist, the recommendations can be relied upon, assuming no other changes have occurred. *The geotechnical engineer who prepared this report cannot assume responsibility or liability for confirmation-dependent recommendations if you fail to retain that engineer to perform construction observation.*

This Report Could Be Misinterpreted

Other design professionals' misinterpretation of geotechnical-engineering reports has resulted in costly problems. Confront that risk by having your geotechnical engineer serve as a full-time member of the design team, to:

- confer with other design-team members,
- help develop specifications,
- review pertinent elements of other design professionals' plans and specifications, and
- be on hand quickly whenever geotechnical-engineering guidance is needed.

You should also confront the risk of constructors misinterpreting this report. Do so by retaining your geotechnical engineer to participate in prebid and preconstruction conferences and to perform construction observation.

Give Constructors a Complete Report and Guidance

Some owners and design professionals mistakenly believe they can shift unanticipated-subsurface-conditions liability to constructors by limiting the information they provide for bid preparation. To help prevent the costly, contentious problems this practice has caused, include the complete geotechnical-engineering report, along with any attachments or appendices, with your contract documents, *but be certain to note conspicuously that you've included the material for informational purposes only*. To avoid misunderstanding, you may also want to note that "informational purposes" means constructors have no right to rely on the interpretations, opinions, conclusions, or recommendations in the report, but they may rely on the factual data relative to the specific times, locations, and depths/elevations referenced. Be certain that constructors know they may learn about specific project requirements, including options selected from the report, *only* from the design drawings and specifications. Remind constructors that they may

perform their own studies if they want to, and *be sure to allow enough time* to permit them to do so. Only then might you be in a position to give constructors the information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions. Conducting prebid and preconstruction conferences can also be valuable in this respect.

Read Responsibility Provisions Closely

Some client representatives, design professionals, and constructors do not realize that geotechnical engineering is far less exact than other engineering disciplines. That lack of understanding has nurtured unrealistic expectations that have resulted in disappointments, delays, cost overruns, claims, and disputes. To confront that risk, geotechnical engineers commonly include explanatory provisions in their reports. Sometimes labeled "limitations," many of these provisions indicate where geotechnical engineers' responsibilities begin and end, to help others recognize their own responsibilities and risks. *Read these provisions closely*. Ask questions. Your geotechnical engineer should respond fully and frankly.

Geoenvironmental Concerns Are Not Covered

The personnel, equipment, and techniques used to perform an environmental study – e.g., a "phase-one" or "phase-two" environmental site assessment – differ significantly from those used to perform a geotechnical-engineering study. For that reason, a geotechnical-engineering report does not usually relate any environmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. *Unanticipated subsurface environmental problems have led to project failures*. If you have not yet obtained your own environmental information, ask your geotechnical consultant for risk-management guidance. As a general rule, *do not rely on an environmental report prepared for a different client, site, or project, or that is more than six months old*.

Obtain Professional Assistance to Deal with Moisture Infiltration and Mold

While your geotechnical engineer may have addressed groundwater, water infiltration, or similar issues in this report, none of the engineer's services were designed, conducted, or intended to prevent uncontrolled migration of moisture – including water vapor – from the soil through building slabs and walls and into the building interior, where it can cause mold growth and material-performance deficiencies. Accordingly, *proper implementation of the geotechnical engineer's recommendations will not of itself be sufficient to prevent moisture infiltration*. Confront the risk of moisture infiltration by including building-envelope or mold specialists on the design team. *Geotechnical engineers are not building-envelope or mold specialists*.



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